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Page 1

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Recommended Descriptions of Geosynthetics Functions, Geosynthetics Terminology, Mathematical and Graphical Symbols

Foreword

This is the fourth edition of the IGS mathematical and graphical symbols document. Since publication of the third edition in February 1996 a number of evolutionary changes (rather than revolutionary changes) have been made to reflect the further development and refinement of geosynthetics terminology. This edition will also be placed on the IGS Web Site to provide IGS members with ready access to current geosynthetics descriptions, terminology and mathematical and graphical symbols.

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1. Geosynthetics Functions

Barrier: The use of a geosynthetic material to prevent the migration of liquids or gases.

Containment: The use of a geosynthetic material to contain soil or sediments to a specific geometry and prevent its loss. The contained fill takes the shape of the inflated at-rest geometry of the geosynthetic container.

Drainage (a.k.a. transmission): The use of a geosynthetic material to collect and transport fluids.

Filtration: The use of a geosynthetic material to allow passage of fluids from a soil while preventing the uncontrolled passage of soil particles.

Protection: The use of a geosynthetic material as a localised stress reduction layer to prevent or reduce damage to a given surface or layer.

Reinforcement: The use of the tensile properties of a geosynthetic material to resist stresses or contain deformations in geotechnical structures.

Separation: The use of a geosynthetic material between two dissimilar geotechnical materials to prevent intermixing.

Surficial erosion control: The use of a geosynthetic material to prevent the surface erosion of soil particles due to surface water run-off and/or wind forces.

2. Geosynthetics Terminology

Bituminous geomembrane: see Geomembrane, bituminous.

Bonded geogrid: see Geogrid, bonded.

Drainage composite: see Geocomposite drain.

Elastomeric geomembrane: see Geomembrane, elastomeric.

Electrokinetic geosynthetic: A composite material which may provide filtration, drainage, reinforcement in addition to electrical conduction.

Extruded geogrid: see Geogrid, extruded.

Geoarmour: A permeable geosynthetic material placed over the surface of the soil, in conjunction with pattern-placed block armour units, to prevent erosion.

Geobar: A polymeric material in the form of a bar, used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geoblanket: A permeable, biodegradable (synthetic or natural) structure placed over the soil for temporary erosion control applications, usually while vegetation is being established.

Geocell: A three-dimensional, permeable, polymeric (synthetic or natural) honeycomb or web structure, made of strips of geotextiles, geogrids or geomembranes linked

web structure, made of strips of geotextiles, geogrids or geomembranes linked alternatingly and used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geocomposite: A manufactured or assembled material using at least one geosynthetic product among the components, used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geocomposite clay liner: An assembled structure of geosynthetic materials and low hydraulic conductivity earth materials (clay or bentonite), in the form of a manufactured sheet, used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geocomposite drain: A prefabricated subsurface drainage product which consists of a geotextile filter skin supported by a geonet or a geospacer.

Geocomposite reinforcement: An assembled structure of dissimilar geosynthetic materials used for soil reinforcement.

Geofoam: A polymeric material which has been formed by the application of the polymer in semi-liquid form, through the use of a foaming agent, and results in a lightweight material with high void content, used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geoform: A three-dimensional, permeable geosynthetic structure, filled with soil or sediment waste such that the fill takes the shape of the inflated geoform.

Geogrid: A planar, polymeric structure consisting of a regular open network of integrally connected tensile elements, which may be linked by extrusion, bonding or interlacing, whose openings are larger than the constituents, used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geogrid, bonded: A geogrid manufactured by bonding, usually at right angles, two or more sets of strands or elements.

Geogrid, extruded: A geogrid manufactured by extruding polymers and drawing in a sheet form.

Geogrid, knitted: A geogrid manufactured by knitting together yarns or elements, usually at right angles to each other.

Geogrid, woven: A geogrid manufactured by weaving yarns or elements, usually at right angles to each other.

Geomat: A three-dimensional, permeable, polymeric structure, made of bonded filaments, used to reinforce roots of grass and small plants and extend the erosion-control limits of vegetation for permanent erosion control applications.

Geomattress: A three-dimensional, permeable geosynthetic structure, placed over the surface of a soil, and then filled with concrete mortar or soil, to prevent erosion.

Geomembrane: A planar, relatively impermeable, polymeric (synthetic or natural) sheet used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geomembrane, bituminous: A planar, relatively impermeable sheet manufactured from natural bituminous materials.

Geomembrane, elastomeric: A planar, relatively impermeable sheet manufactured from elastomeric polymers.

Geomembrane, plastomeric: A planar, relatively impermeable sheet manufactured from plastomeric polymers.

Geonet: A planar, polymeric structure consisting of a regular dense network, whose constituent elements are linked by knots or extrusions and whose openings are much larger than the constituents, used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geospacer: A three-dimensional polymeric structure with large void spaces, used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geostrip: A polymeric material in the form of a strip, used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geosynthetic: A planar, polymeric (synthetic or natural) material used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geotextile: A planar, permeable, polymeric (synthetic or natural) textile material, which may be nonwoven, knitted or woven, used in contact with soil/rock and/or any other geotechnical material in civil engineering applications.

Geotextile, knitted: A geotextile produced by interlooping one or more yarns, fibres, filaments or other elements.

Geotextile, nonwoven: A geotextile in the form of a manufactured sheet, web or batt of directionally or randomly orientated fibres, filaments or other elements, mechanically and/or thermally and/or chemically bonded.

Geotextile, woven: A **geotextile** produced by interlacing, usually at right angles, two or more sets of yarns, fibres, filaments, tapes or other elements.

Knitted geogrid: see Geogrid, knitted.

Knitted geotextile: see **Geotextile**, knitted.

Nonwoven geotextile: see **Geotextile**, nonwoven.

Plastomeric geomembrane: see Geomembrane, plastomeric.

Woven geogrid: see Geogrid, woven.

Woven geotextile: see **Geotextile**, woven.

3. Mathematical Symbols

3.1 General symbols

3.1.1 Dimensions

Symbols used for dimensions are:

L length

M mass

t time

T temperature

- dimensionless

3.1.2 Units

m metre

m² square metre

m	square	metre	
m ³	cubic	metre	
km	kilometre = 10	³ m	
mm	millimetre = 10	⁻³ m	
µm	micrometre or micron = 10	⁻⁶ m	
g	gram		
mg	milligram = 10	⁻³ g	
kg	kilogram = 10	³ g	
Mg	megagram = 10	⁶ g = tonne	
s	second		
N	newton		
kN	kilonewton = 10	³ N	
Pa	pascal = N/m	²	
kPa	kilopascal = kN/m	²	
MPa	megapascal = MN/m	²	
J	joule = Nm		
tex	tex = 10	⁻⁶ kg/m = mg/m	
j/kg	tenacity = 10	⁻⁶ N/tex	
°	degree		
%	percent		
-	pure	number	

3.1.3 Prefixes for units

G	giga = 10	⁹
M	mega = 10	⁶
k	kilo = 10	³
c	centi = 10	⁻²
m	milli = 10	⁻³

μ	micro = 10 ⁻⁶
n	nano = 10 ⁻⁹

3.1.4 Recommended subscripts

a	air, active (earth pressure), allowed	
B	base	
cr	creep	reduction
cv	constant volume or critical state	
d	dry state, diameter, design	
f	failure, fibre, filament, final	
GSY	geosynthetic material, e.g.	t_{gsy} is thickness of geosynthetic material
GBA	geobar	
GBL	geoblanket	
GCE	geocell	
GCD	geocomposite	drain
GCL	geocomposite clay liner	
GEC	geosynthetic erosion control material	
GEK	electrokinetic	geosynthetic
GFO	geofoam	
GFR	geoform	
GGR	geogrid	
GMA	geomat	
GMB	geomembrane	
GMT	geomattress	
GNT	geonet	
GSP	geospacer	
GST	geostrip	
GTX	geotextile	
GTXw	woven	geotextile
GTXnw	nonwoven	geotextile
h	horizontal	
i	immediate,	initial
j	joint	
k	characteristic, e.g.	$T_{max,k}$ is characteristic maximum tensile strength
m	material	
max	maximum	

min	minimum	
mr	material	reduction
n	normal,	number
p	passive (earth pressure), planar, pullout	
r	radial,	resistance

req	required	
s	solid particles, sliding	
sat	saturated	
sec	secant	
u	undrained	conditions
v	vertical	
w	water	
x , y	two orthogonal horizontal axes	
z	vertical axis	
	at specific strain or elongation	
0	at rest (earth pressure), zero	
1,2,3	principal	directions

3.1.5 Geometry and kinetics

<i>A</i>	L^2	(m^2)	area	
<i>b</i> , <i>B</i>	<i>L</i>	(m)	breadth or width	
<i>d</i>	<i>L</i>	(m)	diameter	
<i>D</i>	<i>L</i>	(m)	depth	
<i>g</i>	Lt^{-2}	(m/s^2)	acceleration due to gravity $g = 9.8 \text{ m/s}^2$	
<i>H</i>	<i>L</i>	(m)	height	
<i>l</i> , <i>L</i>	<i>L</i>	(m)	length	
<i>t</i>	<i>t</i> (s) time			

t	t (s) time
v	Lt^{-1} (m/s) velocity
V	L^3 (m ³) volume

3.2 Properties related to geosynthetics

3.2.1 Physical properties

$t_{\sigma TX}$	L	(mm)	thickness of GTX, etc.
$b_{\sigma TX}$	L	(m)	width of GTX, etc.
ρ_f	ML^{-3}	(Mg/m ³)	density of fibres or filaments (mass per unit volume)
A	ML^{-2}	(g/m ²)	mass per unit area
d_f	L	(\propto m)	diameter of fibres or filaments
	ML	(tex)	linear density of yarns, fibres, filaments
A	-	(%)	percent open area for wovens or geogrids
$n_{\sigma TX}$	-		porosity (ratio between volume of voids and total volume) of a GTX

3.2.2 Hydraulic properties

O_n	L	(mm, \propto m)	n percent opening size of a GTX - generic term
$O_{n,d}$	L	(mm, \propto m)	n percent opening size as measured by a dry sieving test, $O_{90,d}$ $O_{95,d}$ S ti f d t AOS

			e.g. $O_{90,d}$, $O_{95,d}$. Sometimes referred to AOS or EOS
$O_{95,d}$	L	(mm, \propto m)	Equivalent Opening Size (EOS) corresponding to the 95% opening size measured by a dry sieving test
$O_{n,w}$	L	(mm, \propto m)	n percent opening size as measured by a wet sieving test, e.g. $O_{90,w}$, $O_{95,w}$. Sometimes referred to D_w
q_n	LT^{-1} (litres/m ² .s)		flow capacity normal to the plane of a GTX - generic term.
$q_{n,h}$	LT^{-1} (litres/m ² .s)		flow capacity normal to the plane of a GTX under water head h (in mm), e.g. $q_{n,100}$ is flow capacity normal to the plane under water head of 100mm.
q_p	L^2T^{-1} (litres/m.s)		flow capacity within the plane of a GTX, GNE or GCD - generic term.

$q_{p,i}$	$L^2 T^{-1}$	(litres/m.s)	generic term. flow capacity within the plane of a GTX, GNE or GCD under hydraulic gradient i , e.g. $q_{p,i}$ is flow capacity the plane under hydraulic gradient of 1.
k_n	$L t^{-1}$	(m/s)	coefficient of permeability normal to the plane
k_p	$L t^{-1}$	(m/s)	coefficient of permeability in the plane of a GTX or GCD
	t^{-1}	(s ⁻¹)	permittivity of a GTX. $= k_n / t_{\sigma\sigma}$
	$L^2 t^{-1}$	(m ² /s)	transmissivity of a GTX or GCD. $= k_p t_{\sigma\sigma}$
	t^{-1}	(s ⁻¹)	permittivity of a GMB to vapour flow (permeance). It is the rate of vapour transmission divided by the vapour pressure difference across the GMB.
k'_n	$L t^{-1}$	(m/s)	vapour permeability of a GMB normal to its plane. $k'_n = \text{permeance}$

3.2.3 Mechanical properties

	-	(%)	strain or elongation
	t^{-1}	(%/s)	strain rate
ϵ_f	-	(%)	strain or elongation at failure
ϵ_{max}	-	(%)	maximum strain or elongation
T	$M t^{-2}$	(kN/m)	tension (tensile strength per unit width)
T_x	$M t^{-2}$	(kN/m)	tension at a given elongation x ; e.g. T_{30} is t 30% elongation
T_f	$M t^{-2}$	(kN/m)	tension at failure
T_{max}	$M t^{-2}$	(kN/m)	maximum tension
T_a	$M t^{-2}$	(kN/m)	allowable tension
T_B	$M t^{-2}$	(kN/m)	base tension in a geosynthetic reinforcement after allowing for the effects of creep. Sometimes referred to as creep-limited strength
T_{req}	$M t^{-2}$	(kN/m)	required tension
J	$M t^{-2}$	(kN/m)	tensile stiffness
J_t	$M t^{-2}$	(kN/m)	tangential tensile stiffness at elongation t
J_i	$M t^{-2}$	(kN/m)	initial tensile stiffness (at $t = 0$)
J_{sec}	$M t^{-2}$	(kN/m)	secant tensile stiffness between the origin and elongation t ; e.g. $J_{sec,30}$ is the secant tensile stiffness between elongation $t = 0$ and $t = 30\%$
$J_{sec,n}$	$M t^{-2}$	(kN/m)	secant tensile stiffness between $t = 0$ and $t = n\%$ elongation.

	$ML^{-1}t^{-2}$	(kN/m ² , kPa) tensile stress at elongation	; e.g. σ_{30} is the tensile stress at 30% elongation
σ_{max}	$ML^{-1}t^{-2}$	(kN/m ² , kPa) maximum tensile stress	
σ_f	$ML^{-1}t^{-2}$	(kN/m ² , kPa) tensile stress at failure	
E	$ML^{-1}t^{-2}$	(kN/m ² , kPa) elastic modulus	
E_i	$ML^{-1}t^{-2}$	(kN/m ² , kPa) initial tangential modulus (see J_i)	
E	$ML^{-1}t^{-2}$	(kN/m ² , kPa) tangential modulus at elongation	(see J)
E_{sec}	$ML^{-1}t^{-2}$	(kN/m ² , kPa) secant modulus between the origin and elongation	(see J_{sec})
-		poisson's ratio	
γ	L^2t^2	(N/tex)	tenacity of a yarn (ratio between tensile strength of a yarn and its linear density)
	(varies)		mechanical efficiency (ratio between maximum strength and mass per unit area)
F_f	MLt^{-2}	(N, kN)	load recorded at failure in a tensile test (NB: the tensile test must be specified)
F_{max}	MLt^{-2}	(N, kN)	maximum tensile force of a GT or GM (NB: the tensile test must be specified)
F_G	MLt^{-2}	(N, kN)	breaking force as measured in a Grab test (NB: the Grab test must be specified)
F_P	MLt^{-2}	(N, kN)	breaking force in a static puncture test (NB: the static puncture test must be specified)
F_T	MLt^{-2}	(N, kN)	breaking force in a tear propagation test (NB: the tear propagation test must be specified)
O_d	L	(mm)	perforation resistance in a dynamic tear initiation test (NB: the tear initiation test must be specified)
P_r	Mt^{-2}	(kN/m)	pullout resistance
P_B	$ML^{-1}t^{-2}$	(kN/m ² , kPa) bursting pressure (NB: the burst test must be specified)	
W_i	ML^2t^{-2}	(Joules)	energy measuring the resistance in an impact test (NB: the impact test must be specified)

3.2.4 Interface properties

f_{GST}	-	(-)	friction interaction coefficient between soil and GSY.
f_{GST}	$\tan \phi'$	$\phi' = \tan^{-1} f_{GST}$	ϕ' is friction angle of soil.

$\alpha_{\theta GST}$	-	(-)	coefficient of friction between soil and GSY.
θ_{GST}	-	(°)	effective friction angle between soil and GSY - general term.
ρ, θ_{GST}	-	(°)	effective peak friction angle between soil and GSY.
σ, θ_{GST}	-	(°)	effective large strain friction angle between soil and GSY.

3.3 Properties related to fluids

3.3.1 Physical properties

ρ_w	ML ⁻³	(Mg/m ³)	density of water (mass per unit volume)
γ_w	ML ⁻² t ⁻²	(kN/m ³)	unit weight of water (weight per unit volume)
η_w	ML ⁻¹ t ⁻¹	(kg/ms)	dynamic viscosity of water

3.3.2 Flow properties

h	L	(m)	hydraulic head or potential
Q	L ³ t ⁻¹	(m ³ /s)	rate of discharge (also called flow rate) - volume of water passing through a given area per unit of time
v	Lt ⁻¹	(m/s)	discharge velocity
i	-		hydraulic gradient
j	ML ⁻² t ⁻²	(kN/m ³)	seepage force per unit volume (force per unit volume of a porous medium generated by action of fluid upon the solid elements of the porous medium).

$j = i \gamma_w$

3.4 Properties related to geotechnics

3.4.1 Physical properties

3.4.1.1 Solid particles and their distribution

ρ_s	ML ⁻³	(Mg/m ³)	density of solid particles (ratio between mass and volume of solid particles)
γ_s	ML ⁻² t ⁻²	(kN/m ³)	unit weight of solid particles (weight of solid particles per

γ_s	ML ⁻¹ t	(kN/m ³)	unit weight of solid particles (weight of solid particles per unit volume). $\gamma_s = \frac{W_s}{V_s} g$
d	L	(\propto m, mm) particle diameter	
d_n	L	(\propto m, mm)	n percent diameter (diameter corresponding to weight of finer particles)
C_u	- uniformity coefficient.		$C_u = \frac{d_{60}}{d_{10}}$
3.4.1.2 Density of soils			
γ	ML ⁻³	(Mg/m ³)	density of soil (ratio between total mass and total volume of soil)
γ	ML ⁻² t ⁻²	(kN/m ³)	unit weight of soil (ratio between total weight and total volume of soil). $\gamma = \frac{W}{V} g$
γ_d	ML ⁻³	(Mg/m ³)	density of dry soil (ratio between mass of solid particles and total volume of soil)
γ_d	ML ⁻² t ⁻²	(kN/m ³)	unit weight of dry soil (ratio between weight of solid particles and volume of soil). $\gamma_d = \frac{W_s}{V} g$
γ_{sat}	ML ⁻³	(Mg/m ³)	density of saturated soil (ratio between total mass and total volume of completely saturated soil)
γ_{sat}	ML ⁻² t ⁻²	(kN/m ³)	unit weight of saturated soil (ratio between total weight and total volume of completely saturated soil).
γ'	ML ⁻³	(Mg/m ³)	density of submerged soil (difference between density of soil and density of water). $\gamma' = \gamma - \gamma_w$
γ'	ML ⁻² t ⁻²	(kN/m ³)	unit weight of submerged soil (difference between unit weight of soil and unit weight of water). $\gamma' = \gamma - \gamma_w = \gamma' g$

3.4.1.3 Voids and water in soils

e	-	(-)	void ratio (ratio between volume of voids and volume of solid particles)
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n	-	(%)	porosity (ratio between volume of voids and total volume of soil)
w	-	(%)	water content (ratio between weight of pore water and

			weight of solid particles)
S_r	-	(%)	degree of saturation (ratio between volume of pore water and volume of voids)
3.4.1.4 Consistency of soils			
w_L	-	(%)	liquid limit (water content of a remoulded soil at transition between liquid and plastic states, determined by a standard laboratory test)
w_P	-	(%)	plastic limit (water content of a remoulded soil at transition between plastic and semi-solid states, determined by a standard laboratory test)
w_s	-	(%)	shrinkage limit (maximum water content at which a reduction of water content will not cause a decrease in volume of the soil mass)
I_P	-	(%)	plasticity index (difference between liquid and plasticity limits)
I_L	-	(%)	liquidity limit, defined as $(w - w_P) / I_P$
I_C	-	(%)	consistency index, defined as $(w_L - w) / I_P$
e_{max}	-	(-)	void ratio in loosest state (maximum void ratio obtainable by a standard laboratory procedure)
e_{min}	-	(-)	void ratio in densest state (minimum void ratio obtainable by a standard laboratory procedure)
I_D	-	(-)	density index (also called 'relative density', $I_D = (e_{max} - e) / (e_{max} - e_{min})$
			R

3.4.2 Stresses in soils

	ML ⁻¹ t ⁻²	(kN/m ² , kPa)	normal stress
σ'	ML ⁻¹ t ⁻²	(kN/m ² , kPa)	normal effective stress. $\sigma' = \sigma - u$
σ'_v	ML ⁻¹ t ⁻²	(kN/m ² , kPa)	normal effective stress acting in a vertical direction
σ'_h	ML ⁻¹ t ⁻²	(kN/m ² , kPa)	normal effective stress acting in a horizontal direction
u	ML ⁻¹ t ⁻²	(kN/m ² , kPa)	pore water pressure
	ML ⁻¹ t ⁻²	(kN/m ² , kPa)	shear stress
	-	(%)	strain

3.4.3 Hydraulic properties

k	Lt ⁻¹	(m/s)	coefficient of permeability (or hydraulic conductivity)
i	-	(-)	hydraulic gradient

3.4.4 Mechanical properties

3.4.4.1 Soil behaviour under compressive strains

C_c	-	(-)	compression index (slope of virgin compression curve in a semi-logarithmic plot)
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C_r	-	(-)	in a semi-logarithmic plot) recompression index (slope of recompression curve in a semi-logarithmic plot).
<hr/>			
C_s	-	(-)	secondary compression index (slope of secondary compression curve in a semi-logarithmic plot).
c_h	$L^2 t^{-1}$	(m^2/s)	vertical coefficient of consolidation (due to pore water movement in horizontal direction)
c_v	$L^2 t^{-1}$	(m^2/s)	vertical coefficient of consolidation (due to pore water movement in vertical direction)
m_v	$M^{-1} L t^2 (m^2/MN)$		coefficient of volume change (in vertical direction)
p	$ML^{-1} t^{-2} (kN/m^2, kPa)$		pre-consolidation pressure (the greatest effective overburden pressure the soil mass has carried in the past)
E	$ML^{-1} t^{-2}$	$(MN/m^2, MPa)$	deformation modulus (ratio between a given normal stress change and the strain change in the same direction, all other stresses being constant)
K'	$ML^{-1} t^{-2}$	$(MN/m^2, MPa)$	elastic bulk modulus. $K' = E/(3 \cdot 6)$
k_s	$ML^{-2} t^{-2}$	(kN/m^3)	modulus of subgrade reaction (ratio between change of vertical stress on a rigid plate placed on the soil, and the corresponding change of vertical settlement of the plate)
T_v	-	(-)	time factor, $T_v = t c_v / d^2$, where t is time and d is length of the drainage path
ν	-	(-)	poisson's ratio (ratio between strain changes perpendicular to and in the direction of a given uniaxial stress change)

3.4.4.2 Soil behaviour under shear strains

τ	$ML^{-1} t^{-2}$	$(kN/m^2, kPa)$	shear strength. $\tau = c + \sigma \tan \phi$
σ_u	$ML^{-1} t^{-2}$	$(kN/m^2, kPa)$	shear strength measured under undrained (total stress) conditions. $\sigma_u = c_u + \sigma_u \tan \phi_u$
σ_d	$ML^{-1} t^{-2}$	$(kN/m^2, kPa)$	shear strength measured under drained conditions. $\sigma_d = c_d + \sigma_d \tan \phi_d$

τ_d	$ML^{-1}t^{-2}$	(kN/m ² , kPa)	shear strength measured under effective stress conditions. $\tau_d = c_d + \tan \phi_d$
τ'_{cv}	$ML^{-1}t^{-2}$	(kN/m ² , kPa)	residual shear strength measured under effective stress conditions. $\tau'_{cv} = c'_{cv} + \tan \phi'_{cv}$
c	$ML^{-1}t^{-2}$	(kN/m ² , kPa)	cohesion
c_u	$ML^{-1}t^{-2}$	(kN/m ² , kPa)	cohesion measured under undrained conditions
c_d	$ML^{-1}t^{-2}$	(kN/m ² , kPa)	cohesion measured under drained conditions
c'	$ML^{-1}t^{-2}$	(kN/m ² , kPa)	cohesion measured under effective stress conditions
c'_{cv}	$ML^{-1}t^{-2}$	(kN/m ² , kPa)	residual cohesion measured under effective stress conditions
G'	$ML^{-1}T^{-2}$	(MN/m ² , MPa)	elastic shear modulus. $G' = E/(2+2\nu)$
ϕ	-	(°)	soil friction angle
ϕ_u	-	(°)	soil friction angle measured under undrained conditions
ϕ_d	-	(°)	soil friction angle measured under drained conditions
ϕ'	-	(°)	soil friction angle measured under effective stress conditions

ϕ'_{cv}	-	(°)	residual soil friction angle measured under effective stress conditions, also termed the critical state friction angle
ψ	-	(°)	soil dilation angle
ψ'	-	(°)	soil dilation angle under effective stress conditions
α	- (-)		coefficient of friction of soil. $\alpha = \tan \phi$

3.5 Properties related to geotechnical structures

3.5.1 Structure dimensions

b, B	L	(m)	breadth of foundation, slope or embankment
D	L	(m)	depth of foundation, depth below toe of slope
h, H	L	(m)	vertical height of wall, slope or embankment

h, H	L	(m)	vertical height of wall, slope or embankment
l, L	L	(m)	length of foundation or embankment
s	L	(m)	settlement
U	-	(%)	degree of consolidation
	-	(°)	angle of slope to horizontal

3.5.2 External applied loads

F_h	MLt ⁻² or Mt ⁻²	(kN or kN/m)	external applied concentrated horizontal force
F_v	MLt ⁻² or Mt ⁻²	(kN or kN/m)	external applied concentrated vertical force
w_s	ML ⁻¹ t ⁻²	(kN/m ² , kPa)	external applied surcharge load

3.5.3 Earth pressures

K	-	(-)	ratio of horizontal to vertical stress
K_a	-	(-)	active earth pressure coefficient
K_o	-	(-)	at-rest earth pressure coefficient
K_p	-	(-)	passive earth pressure coefficient
	ML ⁻¹ t ⁻²	(kN/m ² , kPa)	wall adhesion (adhesion between wall and adjacent soil)
	-	(°)	angle of wall friction (angle of friction between wall and adjacent soil)

3.6 Factors of safety, partial factors and reduction factors

FS	-	(-)	global factor of safety (normally derived from limit equilibrium methods)
α	-	(-)	reduction factor associated with the loss in load carrying capability due to creep effects of a reinforcement over time
f	-	(-)	partial factor associated with dead loads in a structure
q	-	(-)	partial factor associated with live loads in a structure
m	-	(-)	partial factor associated with the strength of the materials used in the structure

m_r	-	(-)	reduction factor associated with the loss in load carrying capability due to installation and durability effects of a reinforcement over time
n	-	(-)	partial factor associated with the economic ramifications of structural failure
p	-	(-)	partial factor associated with the pull-out resistance of geosynthetic reinforcements
s	-	(-)	partial factor associated with the sliding resistance of geosynthetic reinforcements

4. Graphical Symbols

4.1 Products

GTX	Geotextile (generic)
GMB	Geomembrane (generic)
GBA	Geobar (generic)
GBL	Geoblanquet (generic)
GCD	Geocomposite drain (generic) with geotextile on both sides
GCE	Geocell (generic)
GCL	Geocomposite clay liner (generic)
GEC	Surficial geosynthetic erosion control (generic)
GEK	Electrokinetic geosynthetic (generic)
GGR	Geogrid (generic)
GMA	Geomat (generic)
GMT	Geomattress (generic)
GNT	Geonet (generic)
GSP	Geospacer (generic)
GST	Geostrip (generic)

4.2 Functions

The following function symbols may be used where it is considered that a description of the role of the geosynthetic material may provide further clarity to the drawing or diagram.

B Barrier

(fluid)

C Containment (soil & sediments)

D Drainage (fluid)

E Surficial erosion control

F Filtration

P Protection

R Reinforcement

S Separation

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